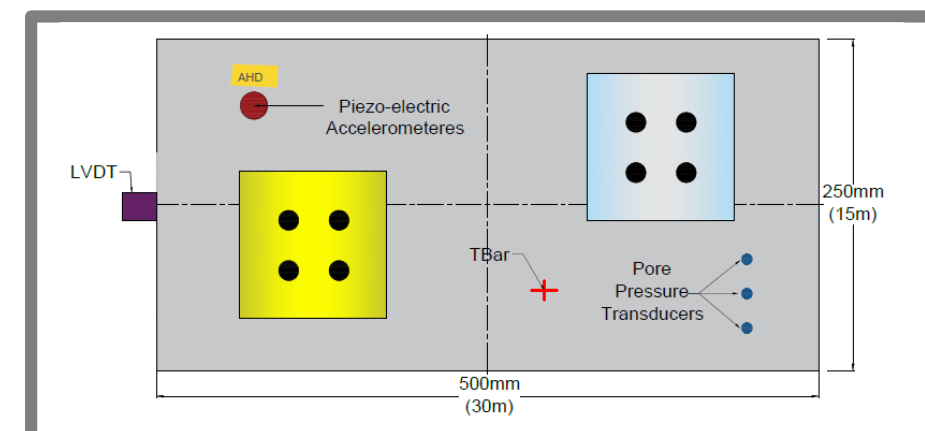
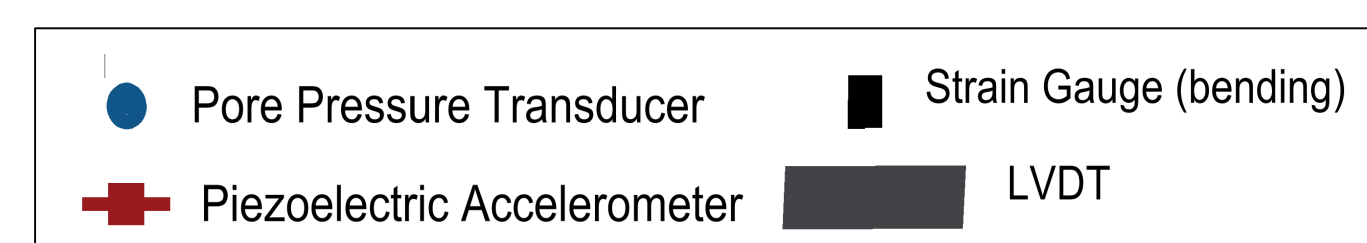


## Background and motivation

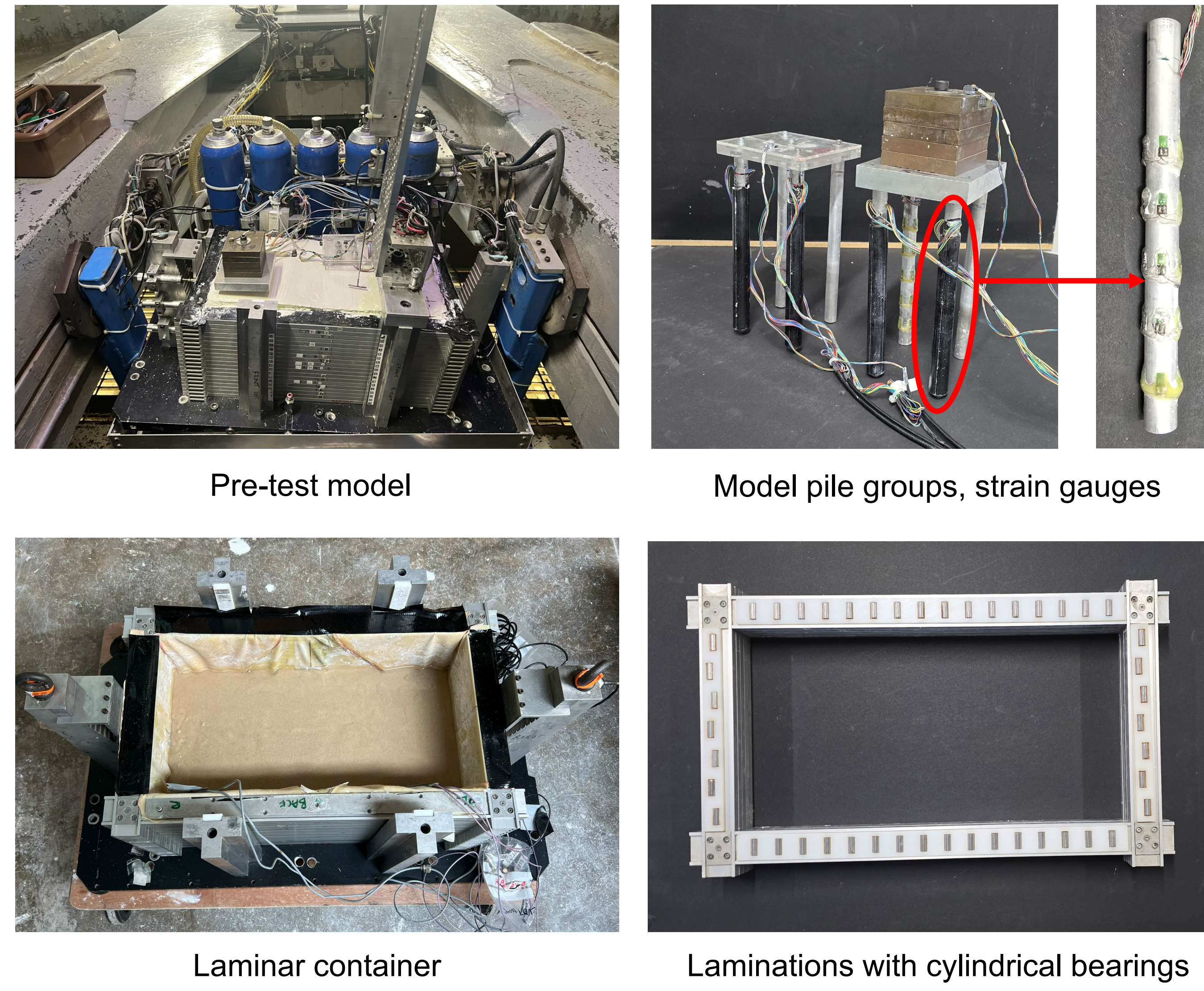
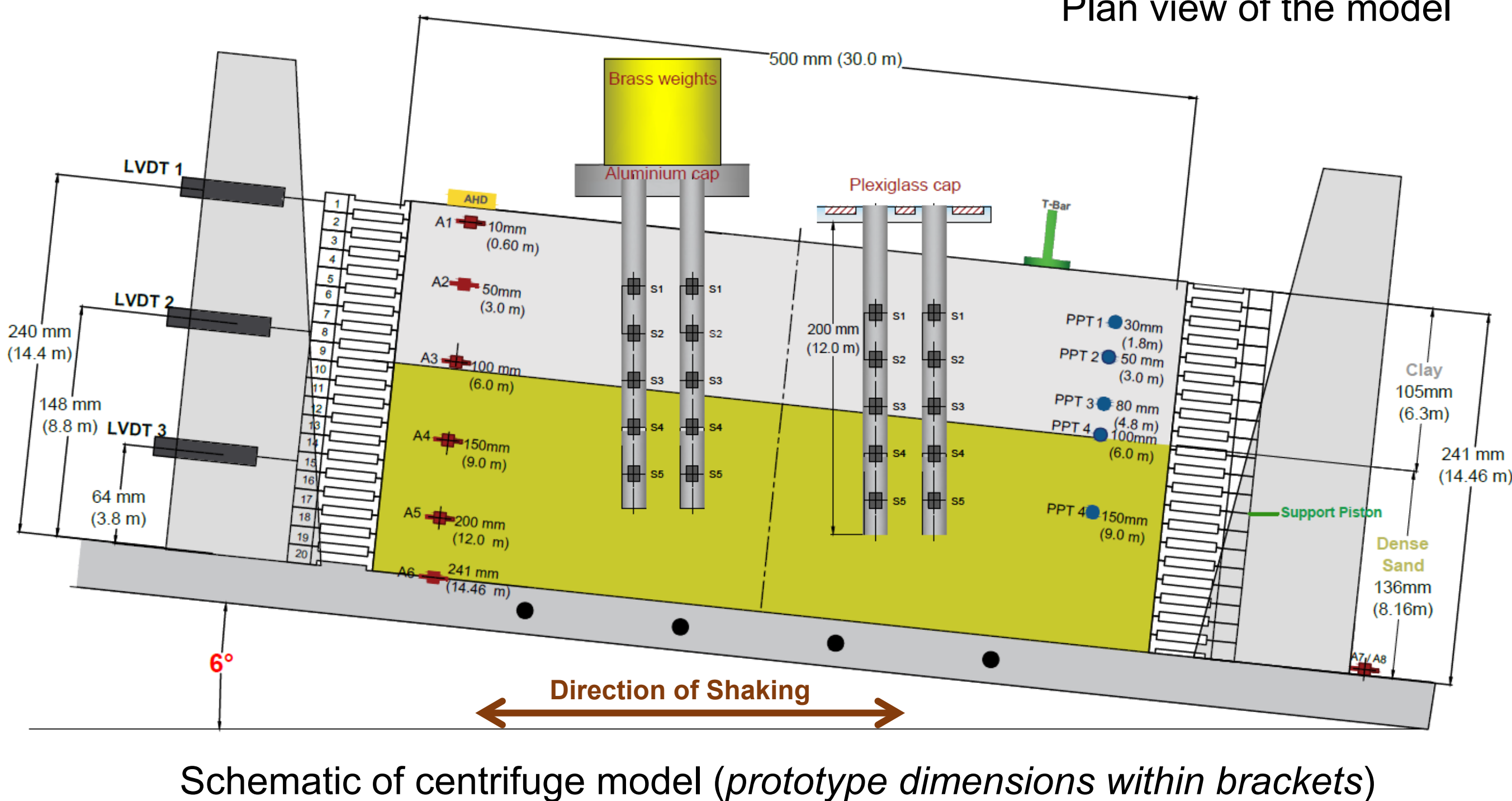
- With the growing global focus on the offshore energy industry, the seismic stability of structures constructed on gentle submarine slopes has become a major concern.
- Submarine slopes** are composed of soft cohesive soils and generally have gentle inclinations. Similar **low-gradient clay slopes** are also common along **riverbanks**, where bridge abutments are often located.
- During an earthquake**, as the **soil moves** downslope, **additional pressures** are exerted on pile foundations built in such slopes (**kinematic interaction / KI**).
- Oscillation of the **superstructural mass** atop the pile generates **extra inertial forces** and overturning moments (**inertial interaction / II**).
- The **simultaneous interaction** of **KI + II** is **complex and sparsely researched**.
- Understanding the dynamic behavior of piles in clay slopes is key to **optimising pile design**, especially for seismically active regions.

## Methodology : Centrifuge testing

- Four geotechnical centrifuge tests at **60g** were conducted on **2x2 pile groups with and without top masses**:
  - SW03**: Pile groups of **spacing 2.5D** embedded in a **6° slope**
  - SW04**: Pile groups of **spacing 5D** embedded in a **6° slope**
  - SW05**: Pile groups of **spacing 2.5D** embedded in a **3° slope**
  - SW06**: Pile groups of **spacing 5D** embedded in a **3° slope**
- Prototype pile dimensions **0.9m (D) x 12m (L)**.
- To simulate **KI** → used an acrylic plexiglass pile cap of negligible weight.
- To simulate **KI+II** → brass masses were added on pile cap (*static vertical force at prototype scale = 8.26 MN*).
- The **laminar container** driven solely by soil deformations was utilised to model the sloping ground.
- A saturated dense sand layer prepared with **Hostun sand ( $I_d=80\%$ )** was overlain by a layer of **Speswhite Kaolin clay ( $s_u = 20$  kPa)**.
- Models were subjected to a **range of earthquakes**.
- Pile groups and sensors were placed in separate vertical planes to minimise boundary effects and possible dynamic interactions.

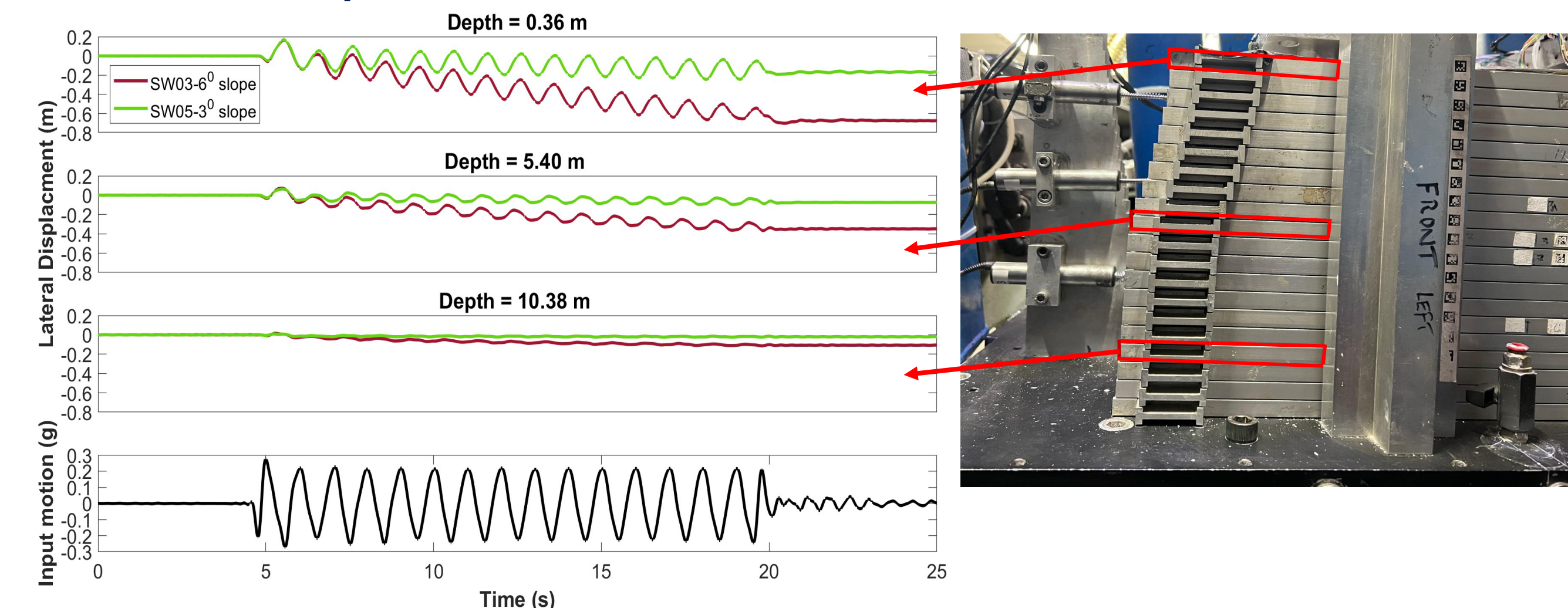


Plan view of the model



## Experimental results

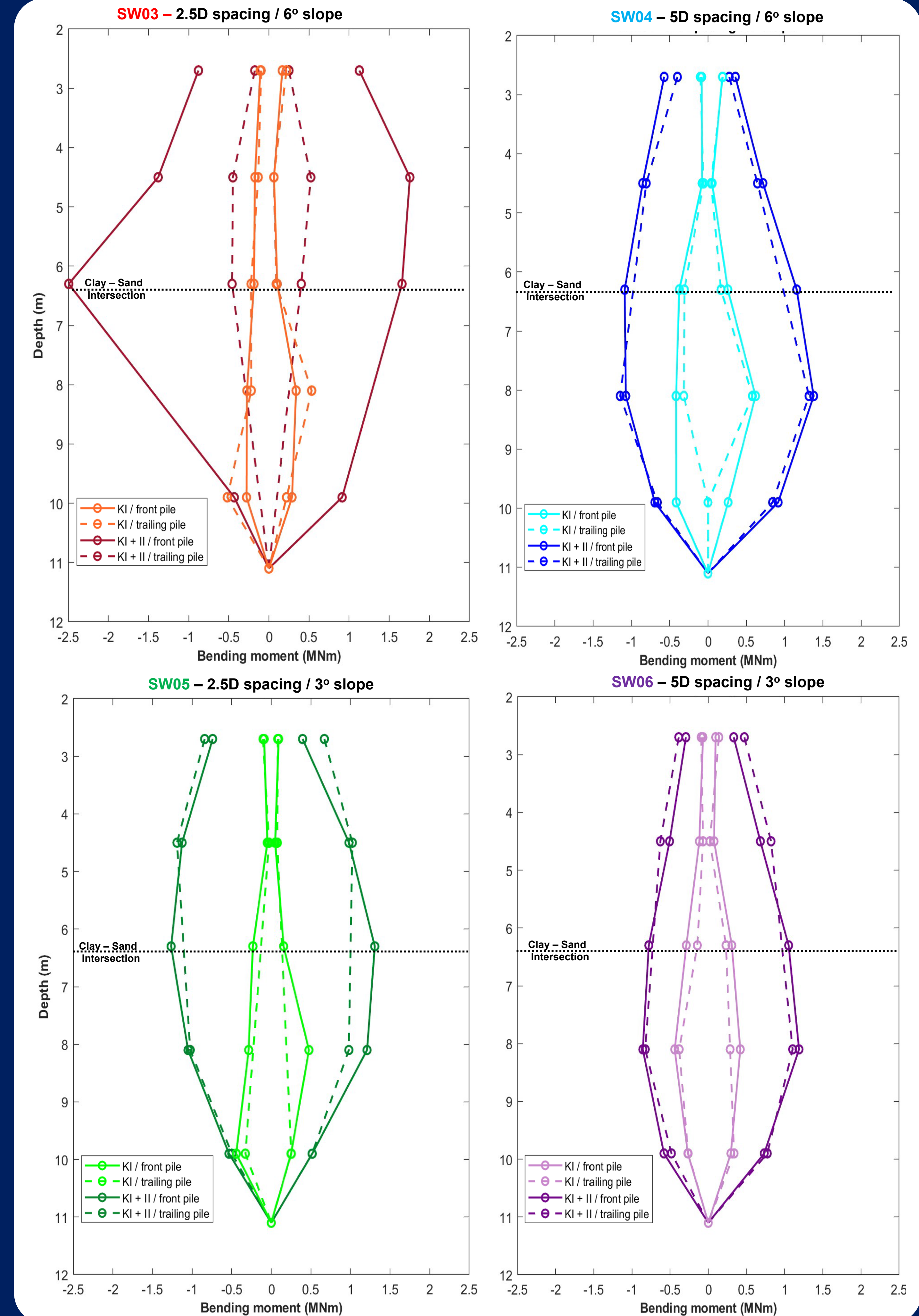
### Lateral soil displacements



Significant lateral displacement was observed in **SW03** compared to **SW05**, as anticipated. Displacement of the 6° slope was approximately 4 times that of the 3° slope.

### Bending moments

- In general, front piles (downslope) experienced greater bending moments than trailing piles (upslope) as they were pushing into undisturbed soil (*solid vs dashed lines*).
- Piles with top mass exhibited significantly higher bending moments, owing to the extra inertial forces generated by the oscillating top mass (*dark vs light shades*).
- Pile groups of 2.5D spacing have seen larger bending moments compared to those of 5D spacing.
- Steeper slopes beset larger bending moments on piles.
- The front and trailing piles in both pile groups with 5D spacing exhibited similar bending moments, regardless of the top mass condition, indicating minimal group effects at this spacing.



## Conclusions and future work

- Steeper slopes generate larger bending moments owing to additional kinematic interaction from slope movement.
- Kinematic interaction has little effect on pile bending moments compared to inertial interaction.
- Pile spacing of a pile group influences the kinematic interaction (i.e shadowing effects) irrespective of top mass conditions.
- Shadowing effects, which are significant at a 2.5D spacing, diminish when the group is more widely spaced at 5D.
- Future work: new design guidelines for more effective pile group design.
  - Quantify the individual kinematic and inertial demands posed on the pile foundations under dynamic and sloping clay ground conditions.
  - Quantify the influence of pile spacing in the dynamic response of pile groups.